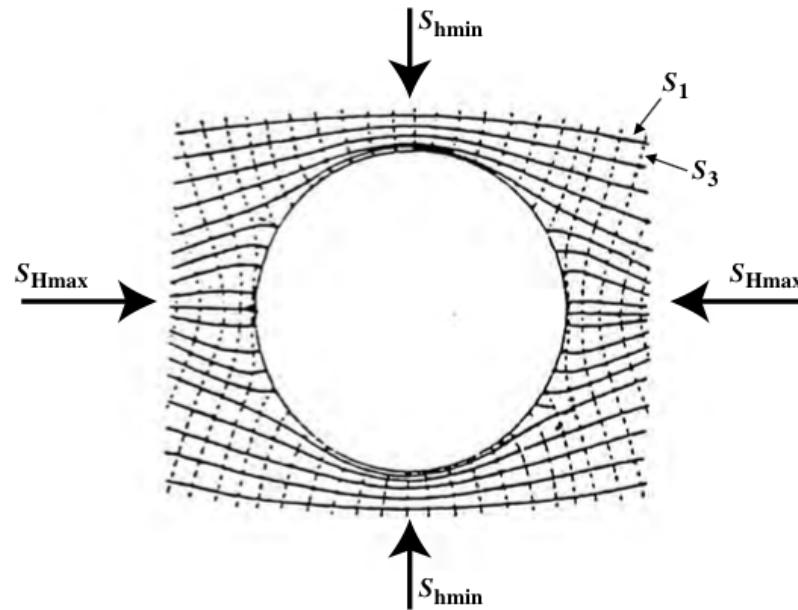


# Compressive and tensile failure in vertical wells

# Stress around circular cavity



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# Kirsch solution

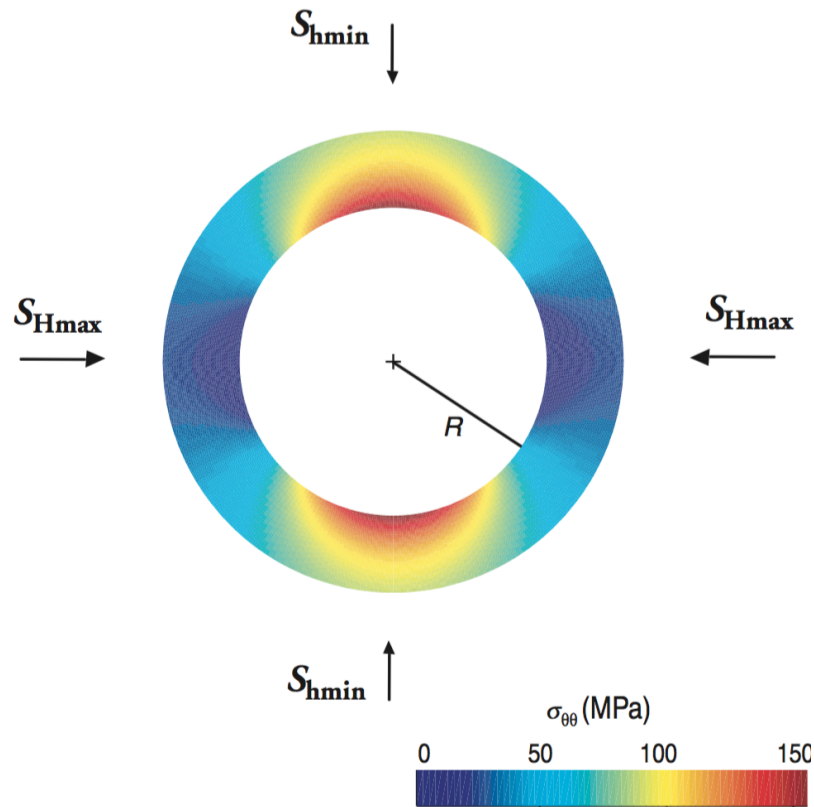
$$\sigma_{rr} = \frac{\sigma_{Hmax} + \sigma_{hmin}}{2} \left(1 - \frac{a^2}{r^2}\right) + \frac{\sigma_{Hmax} - \sigma_{hmin}}{2} \left(1 - 4\frac{a^2}{r^2} + 3\frac{a^4}{r^4}\right) \cos 2\theta + (P_w - P_p) \left(\frac{a^2}{r^2}\right)$$

$$\sigma_{\theta\theta} = \frac{\sigma_{Hmax} + \sigma_{hmin}}{2} \left(1 + \frac{a^2}{r^2}\right) - \frac{\sigma_{Hmax} - \sigma_{hmin}}{2} \left(1 + 3\frac{a^4}{r^4}\right) \cos 2\theta - (P_w - P_p) \left(\frac{a^2}{r^2}\right)$$

$$\sigma_{r\theta} = \frac{\sigma_{Hmax} - \sigma_{hmin}}{2} \left(1 + 2\frac{a^2}{r^2} - 3\frac{a^4}{r^4}\right) - \sin 2\theta$$

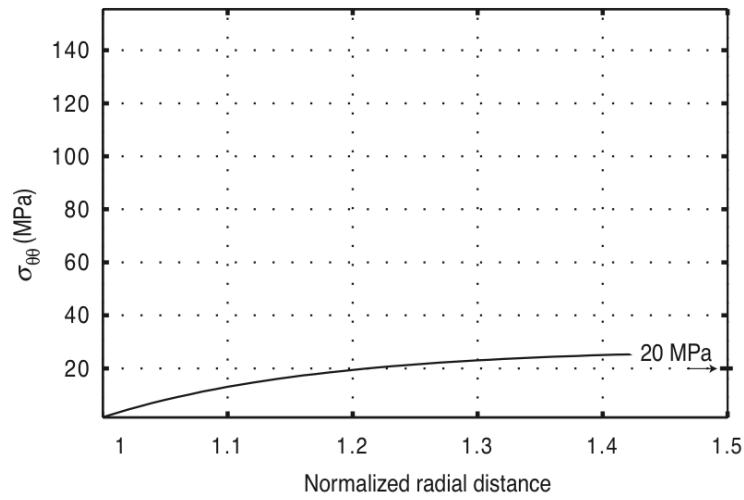
$$\sigma_{zz} = \sigma_v - 2\nu(\sigma_{Hmax} - \sigma_{hmin}) \left(\frac{a^2}{r^2}\right) \cos 2\theta$$

# Example

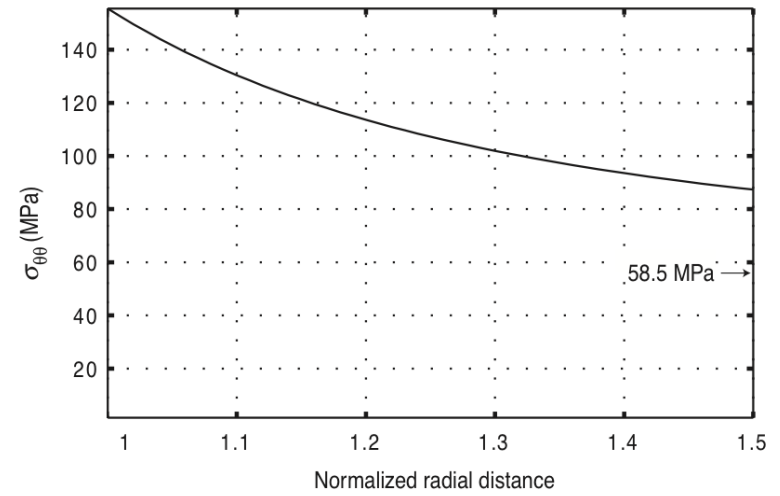


$$\begin{aligned}S_{Hmax} &= 90\text{MPa} \\S_v &= 88.2\text{MPa} \\S_{hmin} &= 51.5\text{MPa} \\P_p &= 31.5\text{MPa} \\P_w &= 31.5\text{MPa}\end{aligned}$$

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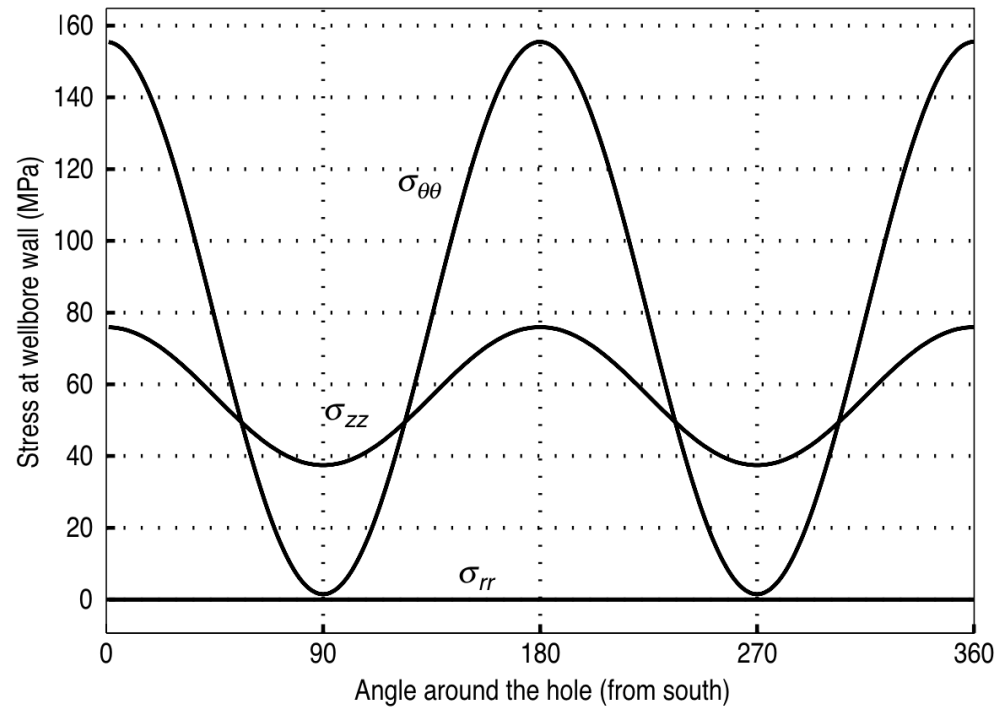
Along azimuth of  $S_{Hmax}$



Along azimuth of  $S_{hmin}$

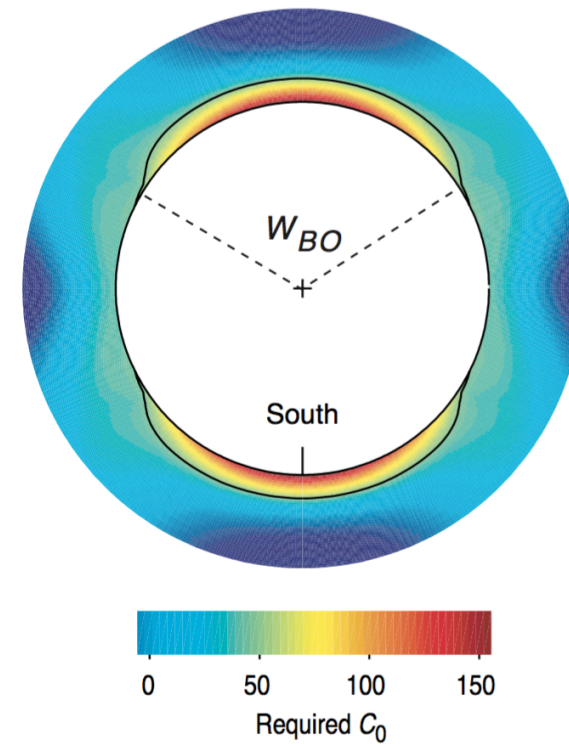
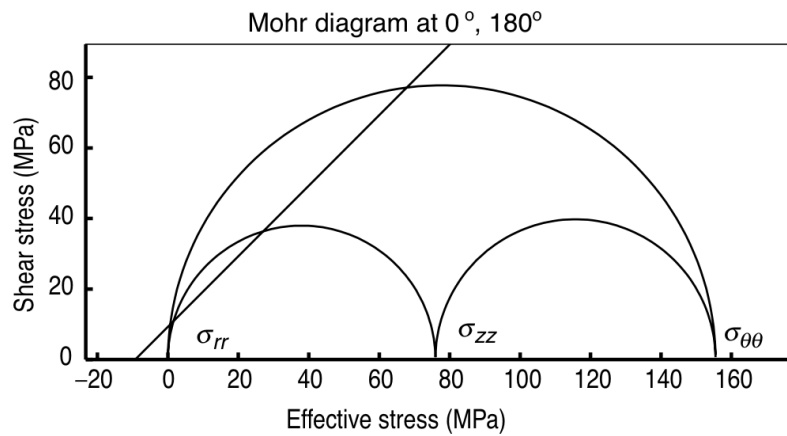
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# Variation of wellbore stresses



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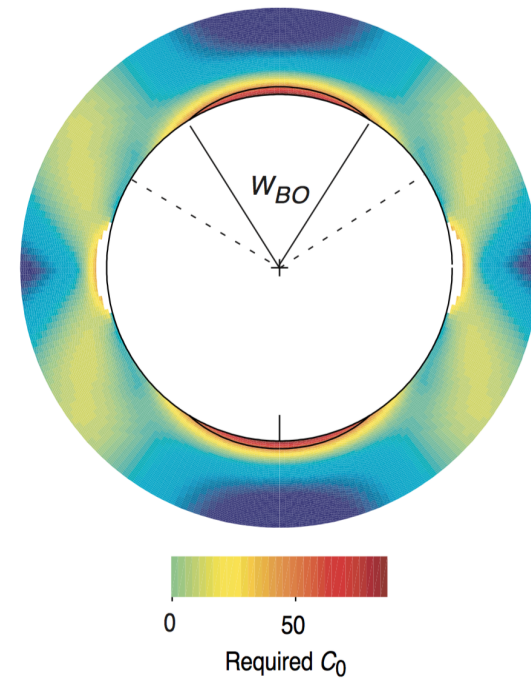
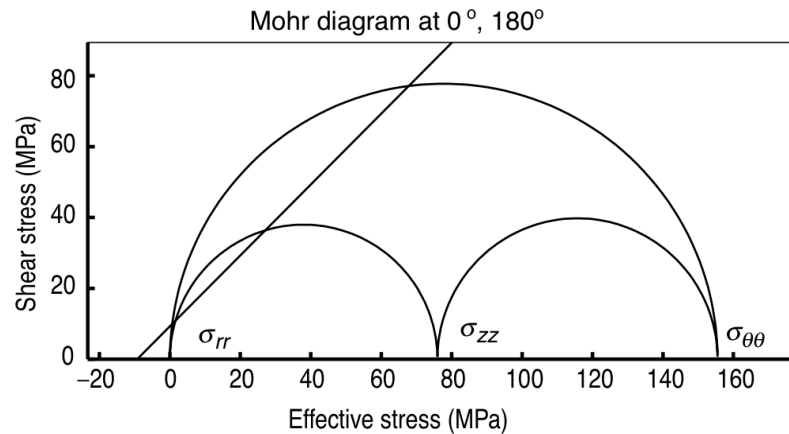
# Wellbore breakout region



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# Mudweight stabilization

As  $\Delta P$  increases,  $\sigma_{\theta\theta}$  decreases and  $\sigma_{rr}$  increases.



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# Breakouts as indicators of far-field stresses

Simplify Kirsch equations at wellbore wall  $a = r$ , so

$$\sigma_{rr} = (P_w - P_p) = \Delta P$$

$$\sigma_{\theta\theta} = \sigma_{Hmax} + \sigma_{hmin} - 2(\sigma_{Hmax} - \sigma_{hmin}) \cos 2\theta - \Delta P$$

$$\sigma_{zz} = \sigma_v - 2\nu(\sigma_{Hmax} - \sigma_{hmin}) \cos 2\theta$$

$\sigma_{\theta\theta}$  has min at  $0^\circ$  and  $180^\circ$

$$\sigma_{\theta\theta}^{min} = 3\sigma_{Hmin} - \sigma_{Hmax} - \Delta P$$

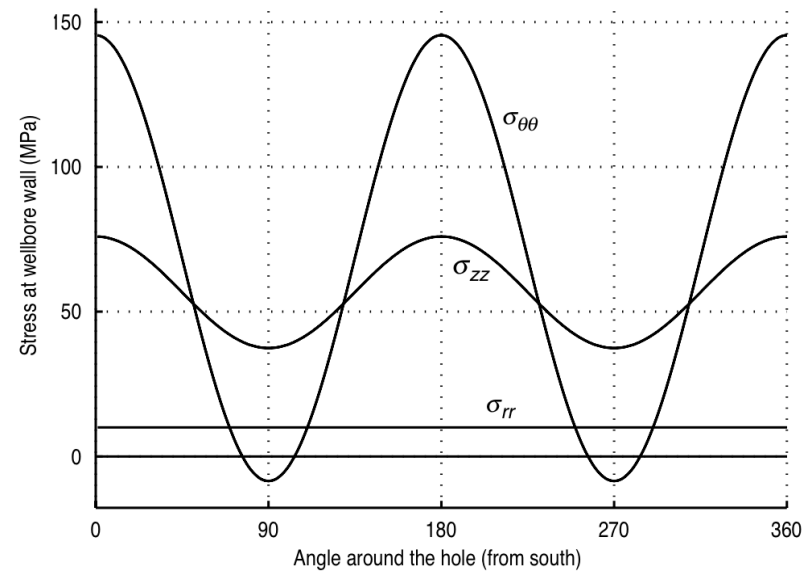
$\sigma_{\theta\theta}$  has min at  $90^\circ$  and  $270^\circ$ , so

$$\sigma_{\theta\theta}^{max} = 3\sigma_{Hmax} - \sigma_{hmin} - \Delta P$$

SO

$$\sigma_{\theta\theta}^{max} - \sigma_{\theta\theta}^{min} = 4(\sigma_{Hmax} - \sigma_{hmin})$$

# Tensile induced fractures

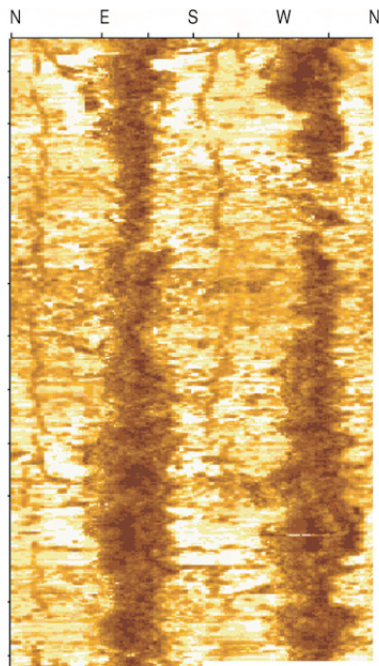


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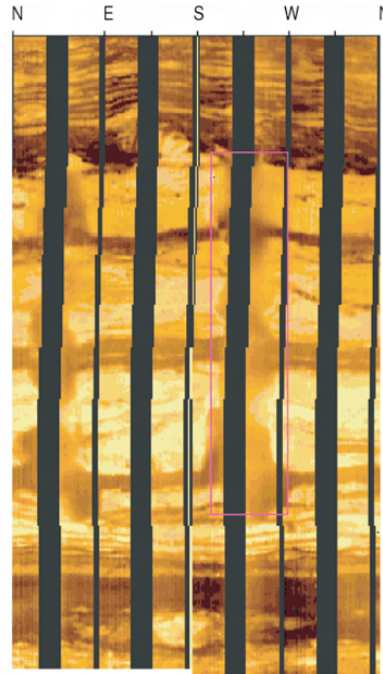
# Safe drilling mud window

- **Mud weight too low**
  - Breakouts
- **Mud weight too high**
  - Tensile induced fractures leading to lost circulation

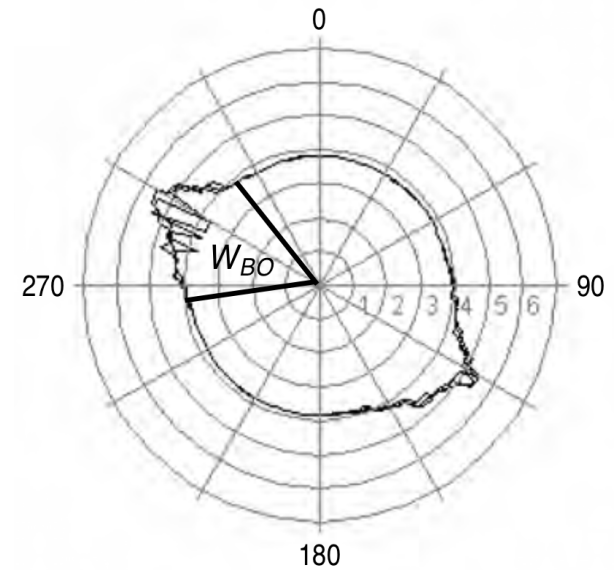
# Imaging breakouts



Ultrasonic  $P$ -wave



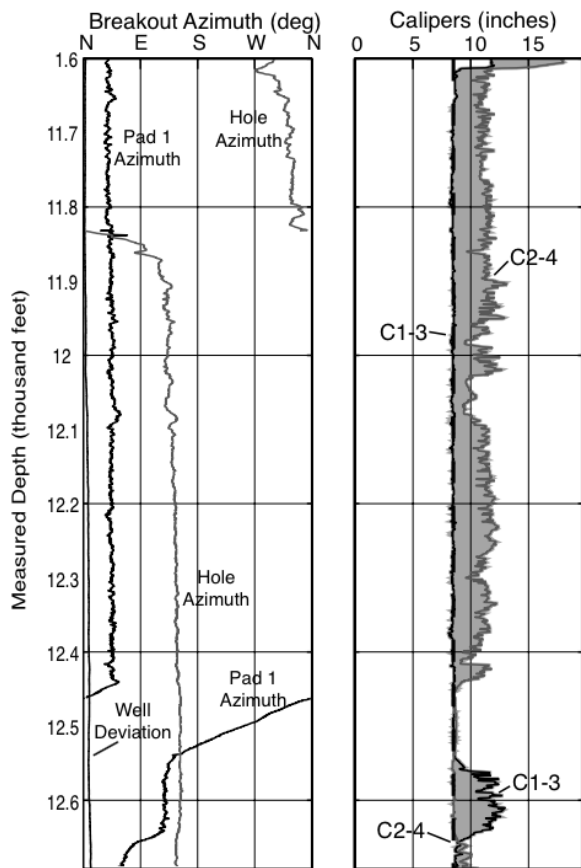
Electrical resistivity



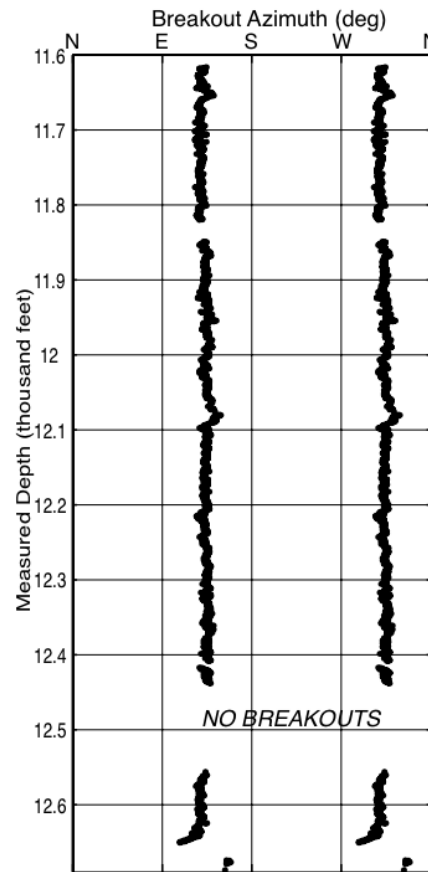
Breakout cross-section

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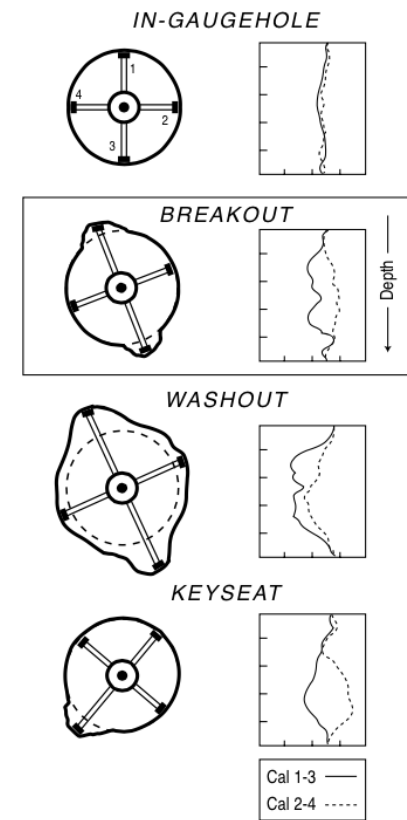
# Four-arm caliper data



Caliper data



Breakout indication



Examples of variations

# Thermal effects on wellbore stress

Strongly time dependent

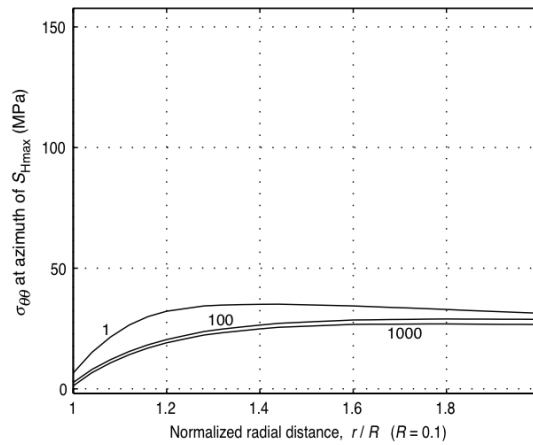
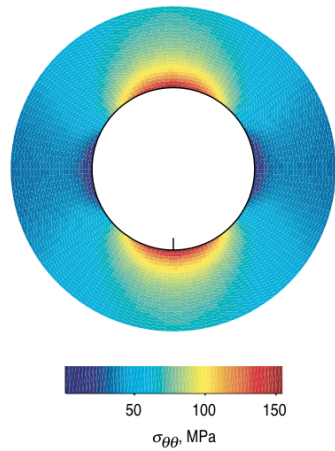
$$\frac{\partial T}{\partial t} = \alpha_T \nabla^2 T$$

$\alpha$  → strongly dependent of the silica content of the rock.

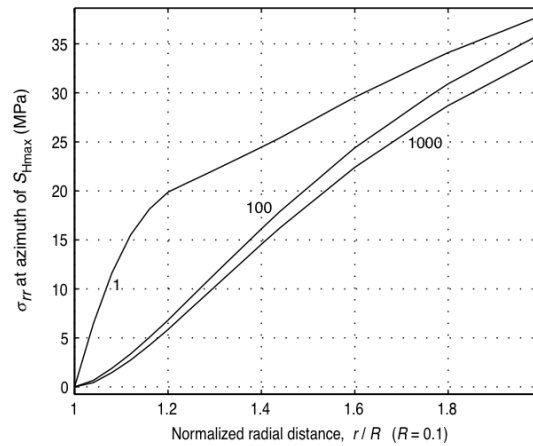
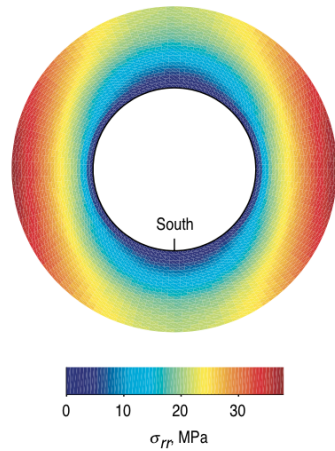
Under steady-state conditions,

$$\Delta\sigma_{\theta\theta}^T = \frac{\alpha_T E \Delta T}{1 - \nu}$$

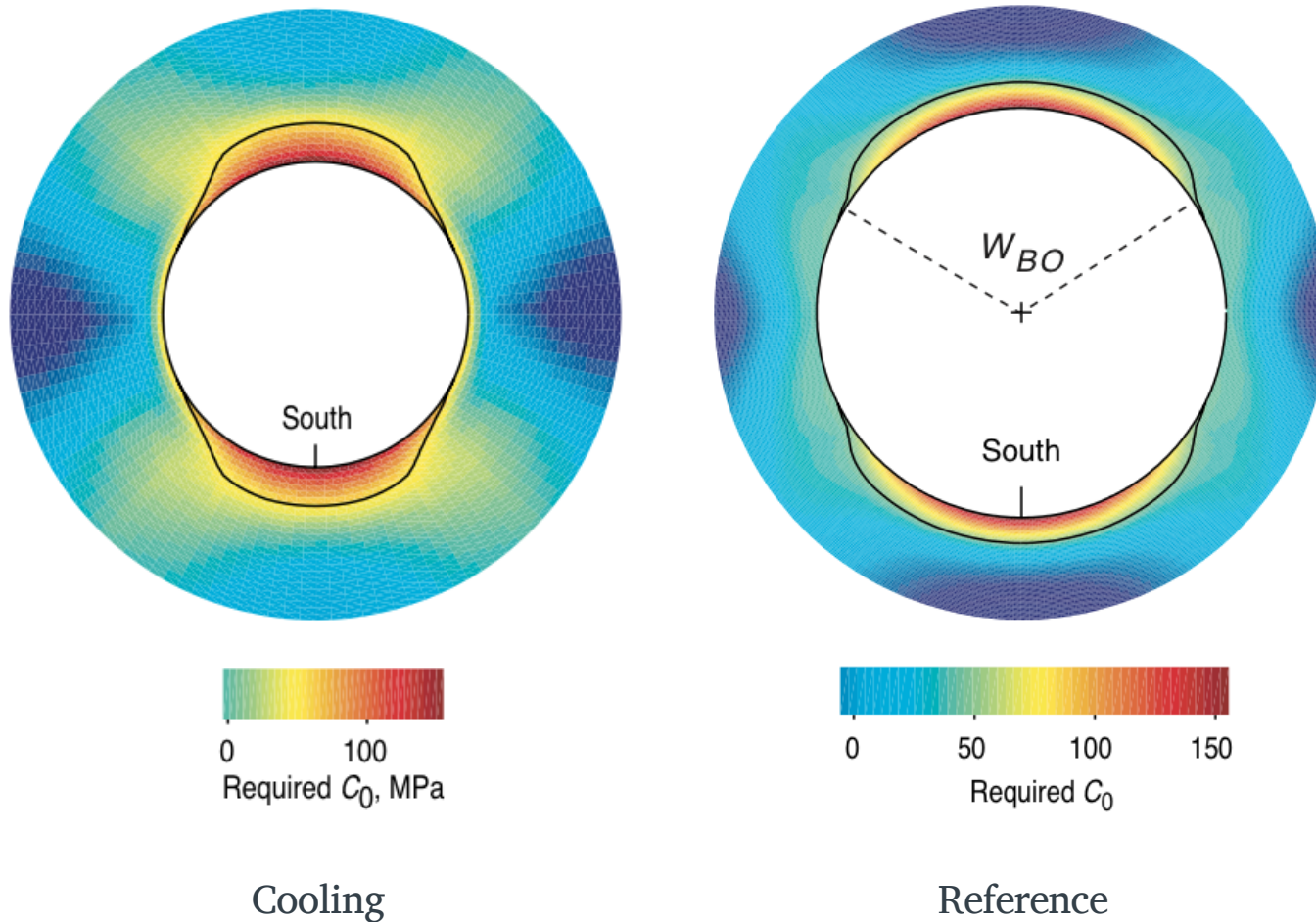
# Time-temperature effects



$\Delta T = 25^\circ \text{ C}$   
 $\Delta P = 6 \text{ MPa}$



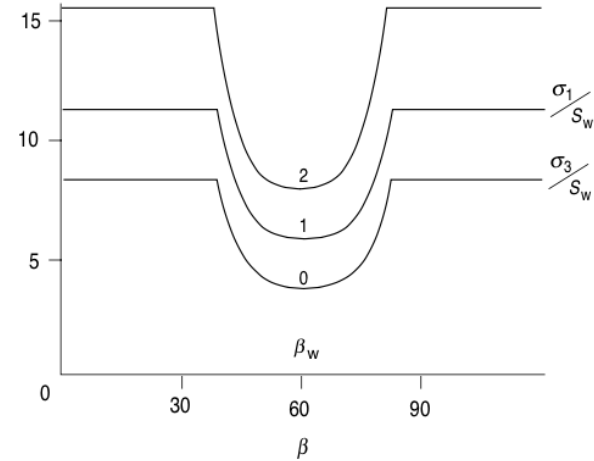
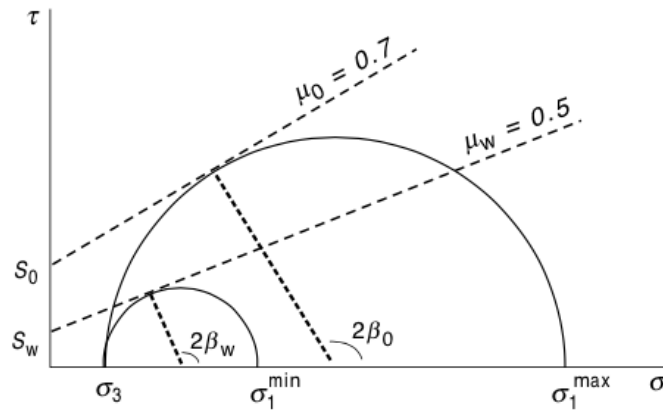
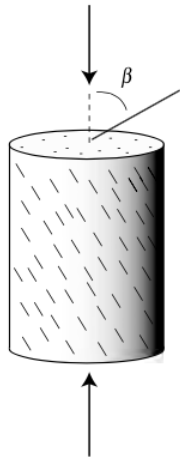
# Stability through cooling?



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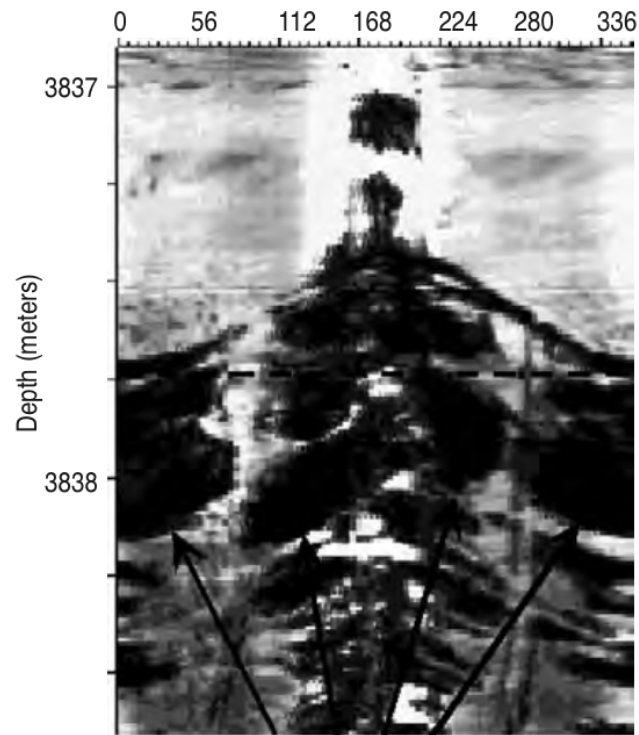
# Rock strength anisotropy



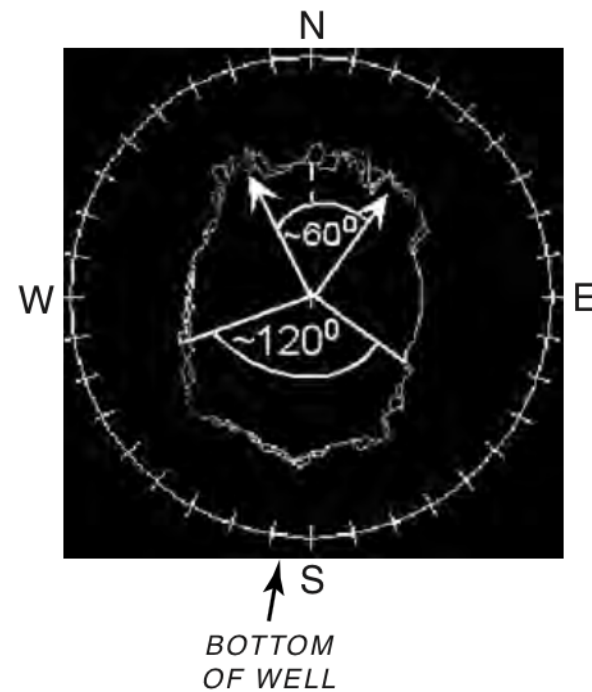
$$\sigma_1 = \sigma_3 \frac{2(S_w + \mu_w \sigma_3)}{(1 - \mu_w \cot \beta_w) \sin 2\beta}$$

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# Rock strength anisotropy effects on breakouts



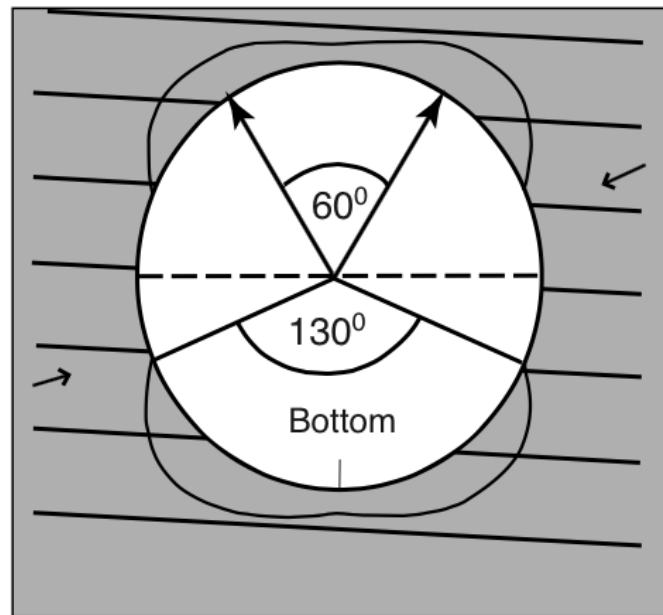
ANISOTROPIC  
BREAKOUT FAILURE



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# Two mechanisms

- Stresses exceed intact rock strength
- Stresses activate slip on weak bedding planes



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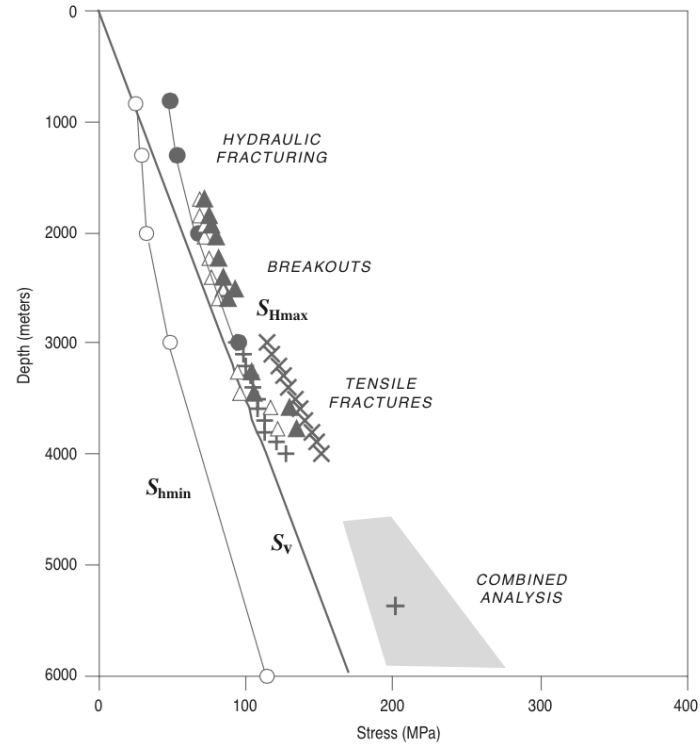
# Chemical effects

- Water Activity ( $A_w \sim \frac{1}{\text{salinity}}$ ) can to increased pore pressure

# $S_{Hmax}$ from breakout data

$$S_{Hmax} = \frac{(C_0 + 2P_p + \Delta P + \Delta\sigma^T) - S_{hmin}(1 + 2\cos(\pi - w_{bo}))}{1 - 2\cos(\pi - w_{bo})}$$

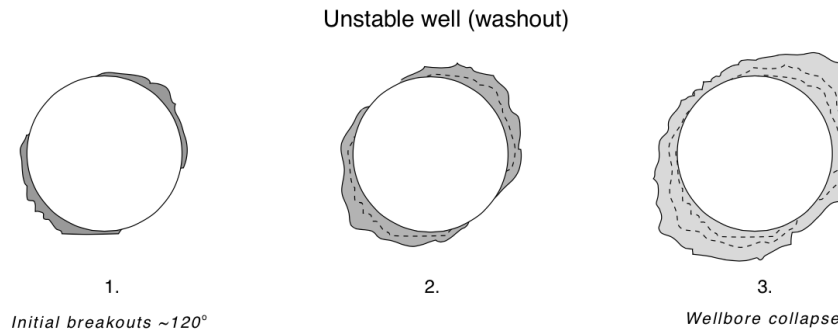
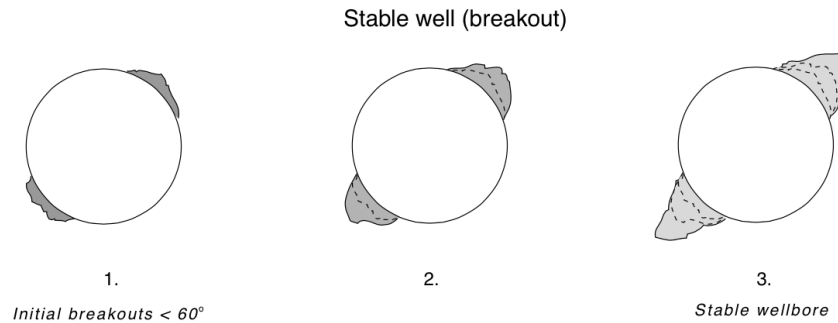
# Example



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# Wellbore stability

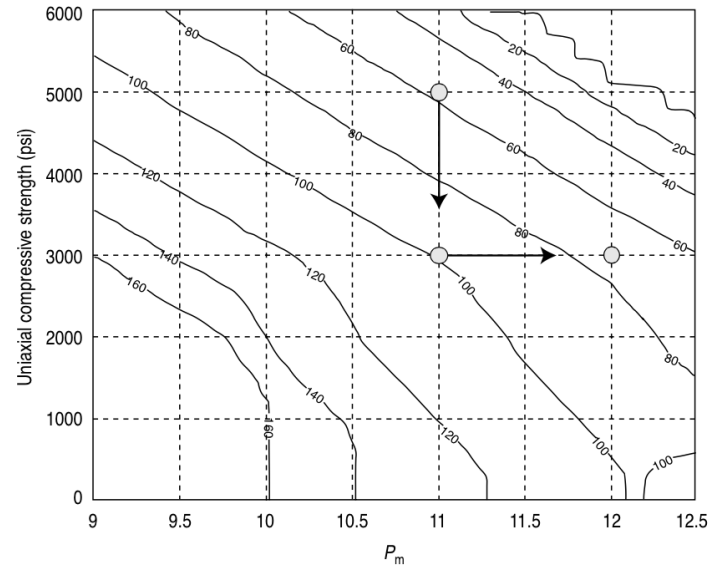
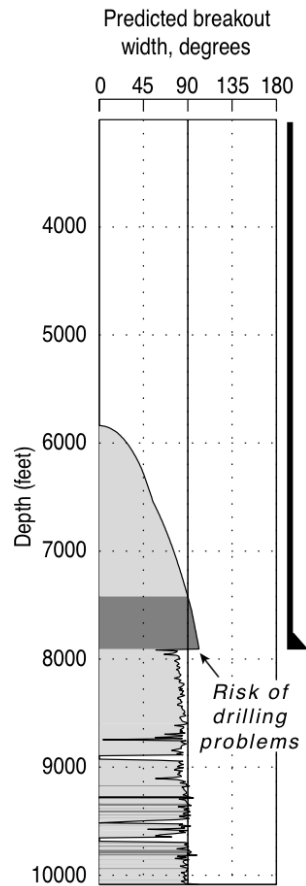
# Defining a "stable" wellbore



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# Emperical model: Maximum 90° breakouts



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# Comprehensive model

i.e. why your studying geomechanics

